B REFERENCE

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**DESCRIPTION** 

LEGEND (SOIL & ROCK)

SUPPLEMENTAL LEGEND (GSI)

BORE LOGS AND CORE LOGS

TITLE SHEET

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CORE PHOTOS SITE PHOTO

SHEET NO.

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5-7

#### STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

## **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY **RICHMOND** 

PROJECT DESCRIPTION REPLACE BRIDGE NO. 164 ON SR 1162 (ALVIN HARRIS RD) OVER HAMER CREEK

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
I.C.	BP8.R002.1	1	13

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THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (1991) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORCHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDING TO CLIMATIC CONDITIONS INCLORDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISTY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

  1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

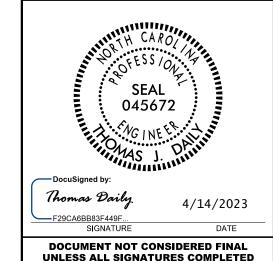
  2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

B. KEBEA J. LITTLE M. RADFORD N. MOHS INVESTIGATED BY S&ME, Inc. DRAWN BY \_C. CHANDLER CHECKED BY \_\_J. DAILY SUBMITTED BY S&ME, Inc.



DATE APRIL 2023

3201 SPRING FOREST ROAD RALEIGH, NC 27616 (919) 872-2660



PROJECT REFERENCE NO. SHEET NO.

BP8.R002.1

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# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

## SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION	UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.	ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.  SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60	AQUIFER - A WATER BEARING FORMATION OR STRATA.
IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING:	GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,	ANGULARITY OF GRAINS	REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
VERY STIFF,GRAY,SILTY CLAY,MOIST WITH INTERBEDDED FINE SAND LAYERS,HIGHLY PLASTIC,A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES >	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	ROCK (WR) 100 BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS	MINERALOGICAL COMPOSITION	CRYSTALLINE FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
CLASS. (\$ 30% PASSING *200) (> 30% PASSING *200)	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.  ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	ROCK (CR) WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 B-2-6 A-2-7 A-1-a-7-5 A-3 A-6, A-7	COMPRESSIBILITY	NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
000000000	SLIGHTLY COMPRESSIBLE LL < 31	ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	OF SLOPE.
SYMBOL 0000000000	MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED
2. PASSING	PERCENTAGE OF MATERIAL	(CP) SHELL BEDS, ETC.	BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
■ 40 30 MX 50 MX 51 MN SOILS CATE PEAT	GRANULAR SILT - CLAY	WEATHERING	<u>DIKE</u> - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
אוח פל אוח פוא פל אוח פ	ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3%, 3 - 5%, TRACE 1 - 10%	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE.	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
MATERIAL PASSING *40	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	HORIZONTAL.
LL 40 MX 41 MN 11TTLE OR	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	(V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
PI 5 MX NP 18 MX 18 MX 11 MN 11 MN 18 MX 18 MX 11 MN 11 MN MODERATE OPENALS	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE  GROUND WATER	OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. $FAULT$ - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
GROUP INDEX 8 8 MX 12 MX 16 MX NU MX AMUUNIS UF SOILS		SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO  (SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
USUAL TYPES STUNE HAUSS. FINE SILTY OR CLAYEY SILTY CLAYEY MATTER		CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SAND SAND GRAVEL AND SAND SOILS SOILS	STATIC WATER LEVEL AFTER 24 HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
GEN. RATING EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE	PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL.
AS SUBURADE PURK	SPRING OR SEEP	WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30		MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH (MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD RANGE OF UNCONFINED PENETRATION RESISTENCE COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION	IF TESTED, WOULD YIELD SPT REFUSAL	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
CUNSISTENCY (N-VALUE) (TONS/FT <sup>2</sup> )	₩ITH SOIL DESCRIPTION → OF ROCK STRUCTURES	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT	ITS LATERAL EXTENT.
GENERALLY VERY LOOSE < 4  CONTROL   VERY LOOSE	SOIL SYMBOL  SOIL SYMBOL  SOIL SYMBOL  SPT DWT TEST BORING  SLOPE INDICATOR INSTALLATION	(SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED  TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
GRANULAR MEDIUM DENSE 10 TO 30 N/A	N N	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS
DENSE   30 TO 50	ARTIFICIAL FILL (AF) OTHER AUGER BORING COME PENETROMETER THAN ROADWAY EMBANKMENT AUGER BORING COME PENETROMETER	VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
VERY SOFT < 2 < 0.25	INFERRED SOIL BOUNDARY - CORE BORING SOUNDING ROD	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK (V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
GENERALLY SOFT 2 TO 4 0.25 TO 0.5	TEST DODING	VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES &lt; 100 BPF</u>	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
SILT-CLAY         MEDIUM STIFF         4 TO 8         0.5 TO 1.0           MATERIAL         STIFF         8 TO 15         1 TO 2	INFERRED ROCK LINE MONITORING WELL WITH CORE	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
(COHESIVE) VERY STIFF 15 TO 30 2 TO 4	TTTTT ALLUVIAL SOIL BOUNDARY A PIEZOMETER SPT N-VALUE	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
HARD > 30 > 4	INSTREERTION	ROCK HARDNESS	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES	ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAV	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	LISED IN THE TOP 3 FEET OF	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER COBBLE GRAVEL COARSE FINE SILT CLAY	UNCLASSIFIED EXCAVATION -  UNDERCUT  UNDERCUT  UNCLASSIFIED EXCAVATION -  EMBANKMENT OR BACKFILL	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
(BLDR.) (COB.) (GR.) (CSE. SD.) (F SD.) (SL.) (CL.)	ABBREVIATIONS	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED	OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.005	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	BY MODERATE BLOWS.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MODE MODERATELY 7 - UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.  HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL
SOIL MOISTURE - CORRELATION OF TERMS	CPT - CONE PENETRATION TEST NP - NON PLASTIC $\dot{\gamma}_{ m d}$ - DRY UNIT WEIGHT	POINT OF A GEOLOGIST'S PICK.	TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION  (ATTERBERG LIMITS) DESCRIPTION	CSE COARSE ORG ORGANIC  DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
	DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK	FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.	STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE	e - VOID RATIO   SD SAND, SANDY   SS - SPLIT SPOON   F - FINE   SL SILT, SILTY   ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH	LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY
LL LIQUID LIMIT	FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK	SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
PLASTIC   SEMISOLID; REQUIRES DRYING TO	FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL		
(PI) PL PLASTIC LIMIT	FRAGS FRAGMENTS	FRACTURE SPACING BEDDING  TERM SPACING TERM THICKNESS	BENCH MARK: BL-IOI N: 511557 E: 1707744
	EQUIPMENT USED ON SUBJECT PROJECT	VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET	ELEVATION: 237.08 FEET
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET	
SL SHRINKAGE LIMIT	X CME-45C CLAY BITS X AUTOMATIC MANUAL	CLOSE Ø.16 TO 1 FOOT VERY THINLY BEDDED Ø.03 - Ø.16 FEET	NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	6' CONTINUOUS FLIGHT AUGER CORE SIZE:	. VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.003 FEET THINLY LAMINATED < 0.008 FEET	
	CME-55   X 8' HOLLOW AUGERS   CORE SIZE:	INDURATION	
PLASTICITY		FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	
PLASTICITY INDEX (PI) DRY STRENGTH NON PLASTIC 0-5 VERY LOW	X CME-550 HARD FACED FINGER BITS X -N Q2 TUNGCARBIDE INSERTS	DIRDING WITH FINCED EDEEC NUMEROUS CRAINS.	
SLIGHTLY PLASTIC 6-15 SLIGHT	I VANE SHEAR TEST │ └──	FRIABLE GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH	X CASING W/ ADVANCER POST HOLE DIGGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;	
COLOR	PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER	BREAKS EASILY WHEN HIT WITH HAMMER.	
LULUN	X TRICONE 2-7/8 TUNGCARB. SOUNDING ROD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	X CORE BIT VANE SHEAR TEST	CHARD HAMMED DIONE DEGITOED TO DREAK CAMPLE.	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		EXTREMELY INDURATED SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-1-

PROJECT REFERENCE NO.	SHEET NO.
BP8.R002.1	2A

DATE: 8-19-16

#### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

## SUBSURFACE INVESTIGATION

SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES

FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000) AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed Rock Mass (Marinos and Hoek, 2000) GEOLOGICAL STRENGTH INDEX (GSI) FOR GSI FOR HETEROGENEOUS ROCK MASSES SUCH JOINTED ROCKS (Hoek and Marinos, 2000) AS FLYSCH (Marinos, P and Hoek E., 2000) From a description of the lithology, structure and athered surf or fillings VERY POOR - Very smooth, slicken-sided or highly weathered surfaces with soft clay coatings or fillings From the lithology, structure and surface and conditions of the discontinuities, estimate the average value of GSI. Do not try to surface conditions (particularly of the bedding smooth, occasionally surfaces with compar fillings with angular planes), choose a box in the chart. Locate the ed filli planes) be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not position in the box that corresponds to the condition of the discontinuities and estimate the average value POOR ensided, highly weather soft clay coatings or of GSI from the contours. Do not attempt to be too apply to structurally controlled failures. Where weak planar structural planes are eq. precise. Quoting a range from 33 to 37 is more highly wea coatings ragments weather slightly realistic than giving GSI = 35. Note that the present in an unfavorable orientation SURFACE CONDITIONS (DISCONTINUITIES (Predominantly beddir Hoek-Brown criterion does not apply to structurally with respect to the excavation face, CONDITIONS these will dominate the rock mass controlled failures. Where unfavourably oriented behaviour. The shear strength of surfaces continuous weak planar discontinuities are present, in rocks that are prone to deterioration slightly es these will dominate the behaviour of the rock mass. Rough, POOR Slickensided, h with compact or angular fre as a result of changes in moisture POOR - Very s slickensided coatings or f fragments The strength of some rock masses is reduced by the **'** 70 content will be reduced if water is **6000** rough, **G00D** thered Smo presence of groundwater and this can be allowed for present. When working with rocks in the by a slight shift to the right in the columns for fair, fair to very poor categories, a shift to the right may be made for wet conditions. th, r FAIR - Weather GOOD Rough, s surface poor and very poor conditions. Water pressure does VERY I VERY Slicke With **VERY** Very FAIR Smoodalter Water pressure is dealt with by effective not change the value of GSI and it is dealt with by stress analysis. using effective stress analysis. COMPOSITION AND STRUCTURE STRUCTURE DECREASING SURFACE QUALITY -> INTACT OR MASSIVE - intact **A.** Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding .90 rock specimens or massive in N/A N/A situ rock with few widely spaced planes is minimized by the confinement of discontinuities the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instabilitu 60 Ы BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets 50 B. Sand-stone wi thin into C. Sand XX D. Siltstone E. Weak 60 or silty shale siltstone stone with stone and R thin inter siltstone with sandor clayey С shale with layers of ın sımıla stone layers VERY BLOCKY - interlocked. OCKING sands tone siltstone 40 partially disturbed mass with 50 multi-faceted angular blocks formed by 4 or more joint sets 퓜  $\mathbf{C}_{\bullet} \, \mathbf{D}_{\bullet} \, \mathbf{E}_{\bullet}$  and  $\mathbf{G}$  - may be more or INTE . Tectonically deformed, BLOCKY/DISTURBED/SEAMY -30 less folded than illustrated but ntensively folded/faulted, folded with angular blocks this does not change the strength. sheared clayey shale or siltstone formed by many intersecting Tectonic deformation, faulting and with broken and deformed CREASING discontinuity sets. Persistence loss of continuity moves these andstone layers forming an 30 categories to F and H. of bedding planes or schistosity Imost chaotic structure 20 DISINTEGRATED - poorly interlocked, heavily broken rock mass 20 G. Undisturbed silty 1. Tectonically deformed silty with mixture of angular and or clayey shale with or clayey shale forming a 10 rounded rock pieces or without a few very chaotic structure with pockets of clay. Thin layers of thin sandstone layers sandstone are transformed nto small rock pieces. 10 LAMINATED/SHEARED - Lack of

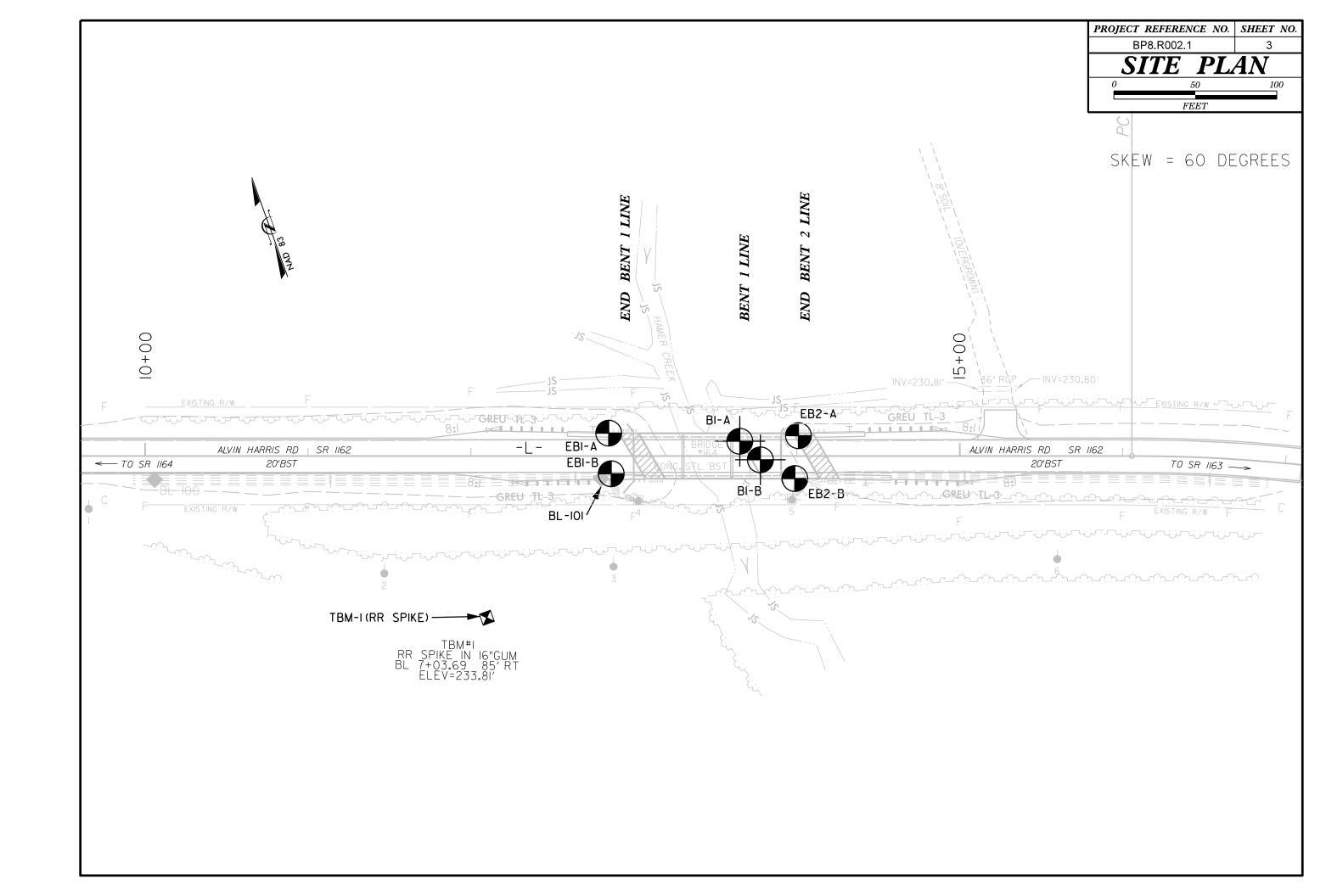
→ Means deformation after tectonic disturbance

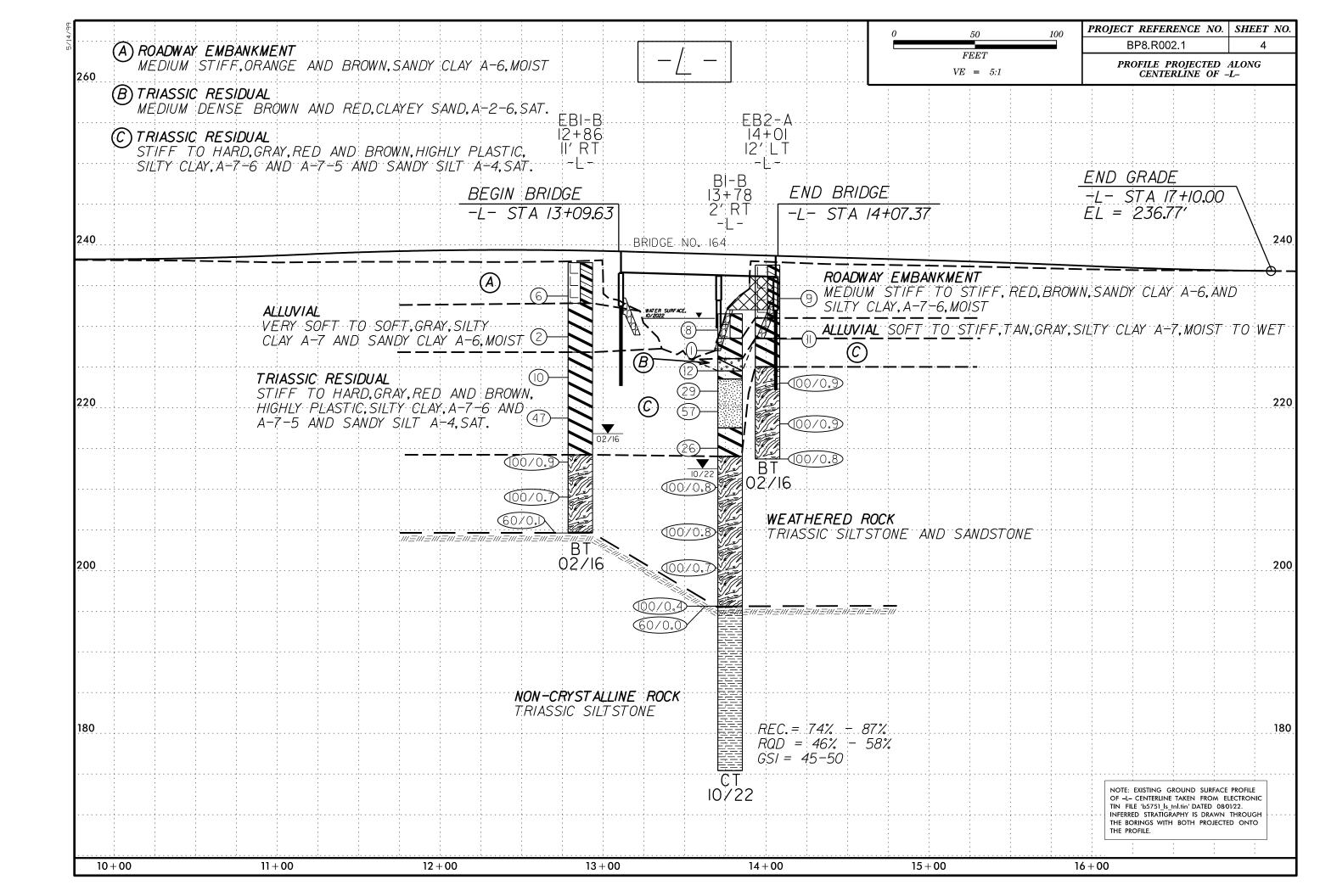
N/A

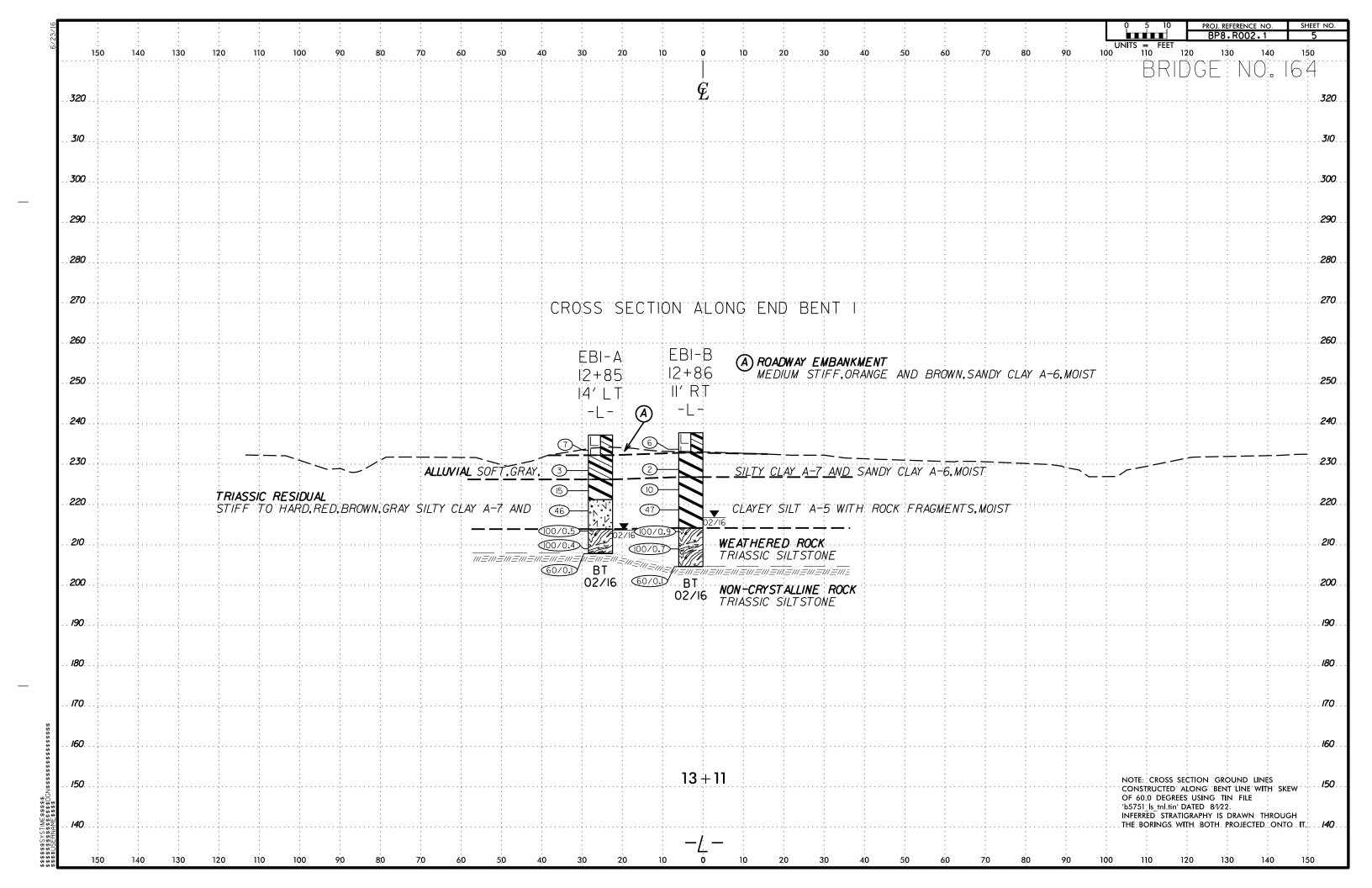
blockiness due to close spacing

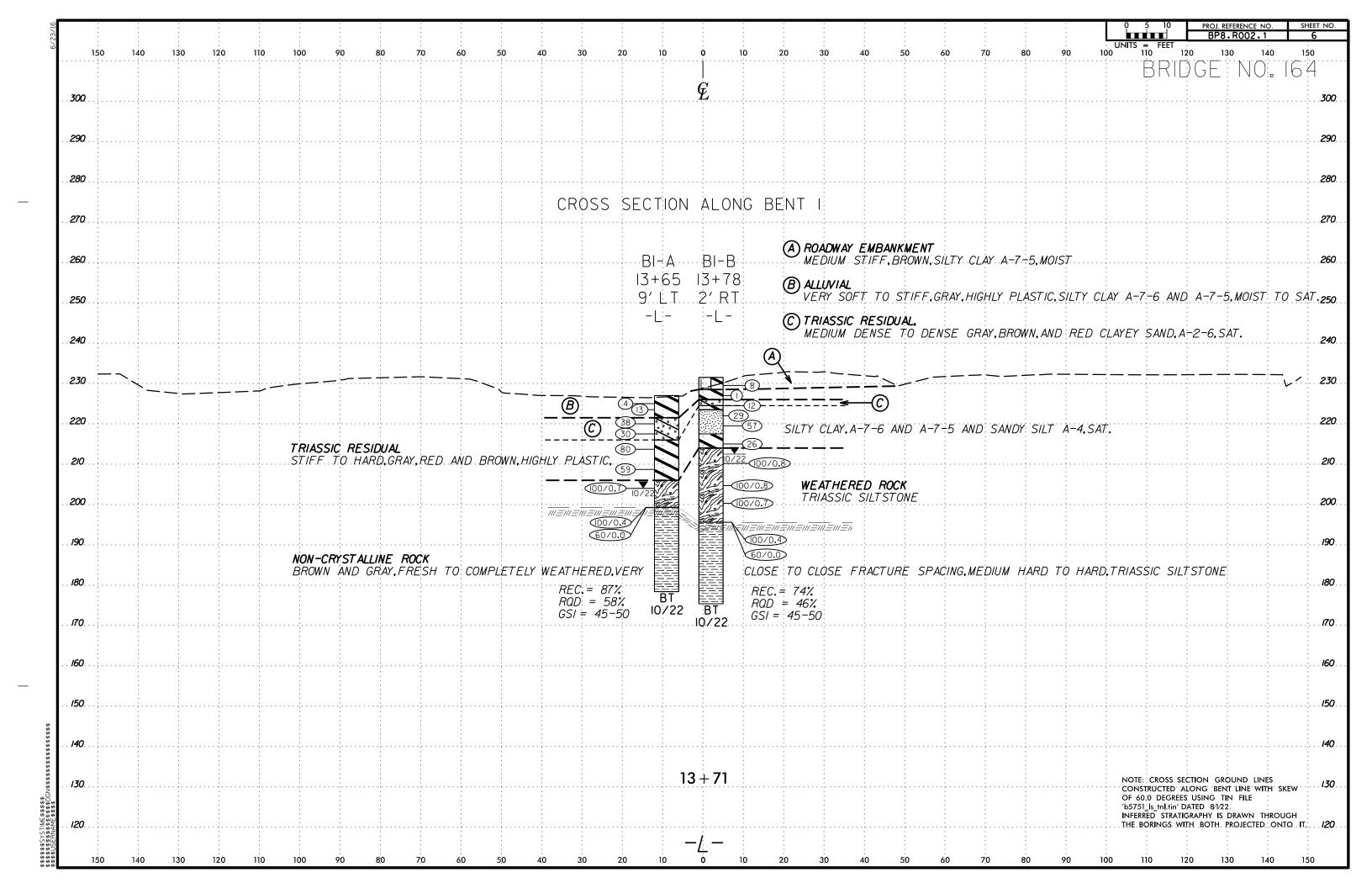
of weak schistosity or shear planes

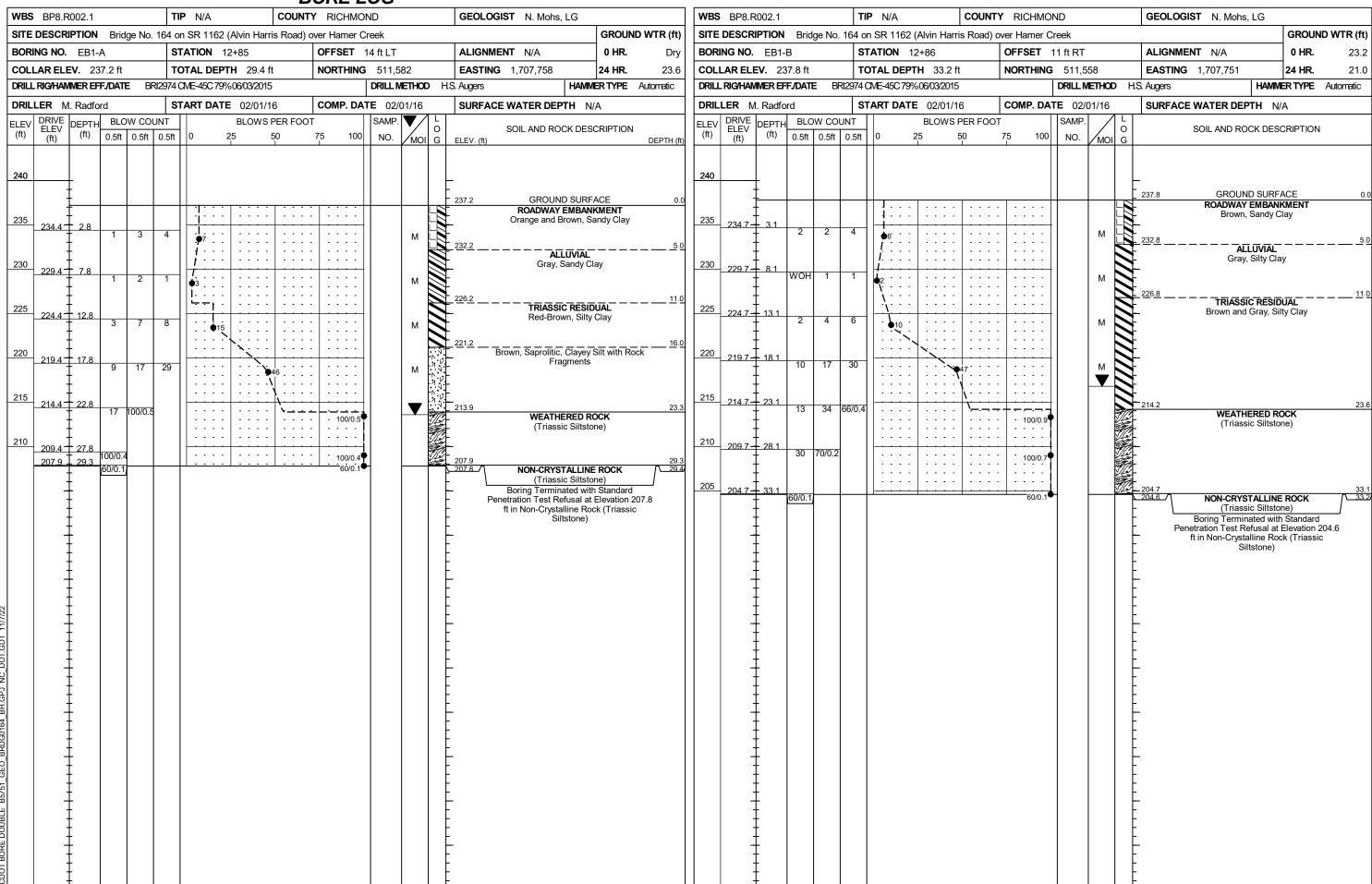
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<b>WBS</b> BP8.R002.1 <b>TIP</b> N/A									COUNT			OG ND			GEOLOGIST Kebea, B.			
SITE DESCRIPTION Bridge No. 164 on SR 1162 (Alvin								(Alvin						(		<u>'</u>	WTR (ft)	
							ATION 13				OFFSET 9 ft LT					ALIGNMENT -L-	0 HR.	N/A
COL	LAR EL	<b>EV.</b> 2:	27.0 ft		Т	01	TAL DEPT	<b>H</b> 48.	6 ft		NOF	RTHING	511,	550		<b>EASTING</b> 1,707,832	24 HR.	23.0
DRIL	L RIG/HA	MMER E	FF./DA	TE S	ME6573	3 (	CME-550X 8	32% 05/	11/202	22	l	DRILL METHOD NW Casing w/ Advancer HAMMER T						utomatic
DRIL	LER L	ittle, J.			s	T/	ART DATE	10/1	2/22		CON	MP. DA	TE 10,	13/22		SURFACE WATER DEPTH	N/A	
ELEV	DRIVE ELEV	DEPTH	BLO	ow co	UNT			BLOW	/S PE	R FOOT			SAMP	<b>V</b> /	L	SOIL AND ROCK DE	SCRIPTION	
(ft)	(ft)	(ft)	0.5ft	0.5ft	0.5ft	Ш	0 2	25 	50		75 	100	NO.	МО		ELEV. (ft)	JONII HON	DEPTH (ft
230		1														_		
		‡														_ 227.0 GROUND SURI	FACE	0.0
225	226.0	1.0	2	2	2	$\parallel$	ļ				Τ.			<b>†</b>		ALLUVIAL		0.0
223	224.5	2.5	6	6	7	+	4				+-			M M		Gray, Silty C -	iay	
	221.0	6.0					: :	[:::			:			"		- - 221.5		5.5
220	- 221.0	- 6.0	20	21	17	1		3	.		<u> </u>			Sat.		- TRIASSIC RESI Gray and Brown, Cla		
	218.5	8.5	3	10	20	$\  \ $		./.	:		:			Sat.	///	- -	, ,	
215		‡						÷30.	`.\.	· · · · . · · ·	:					216.0 Brown and Red, S	illy Clay	11.0
210	214.7 -	12.3	16	29	51	$\exists \dagger$					8	30		Sat.		Blowit and Red, S	only Clay	
		‡									/					- -		
210	209.7	17.3	10	0.5		┨			•	· · ·/:	•					<u>-</u>		
		‡	16	25	34				:	. €59	:			Sat.		<del>-</del> -		
205		‡								: i <u></u> _	.‡-				340	206.0 WEATHERED F	POCK .	21.0
203	204.7	22.3	48	52/0.2	2						†:	100/0.7	,			(Triassic Siltste		
		‡									:					- -		
200	199.7 <del>-</del>	† <del>27.3</del> <del>27.3</del>	100/0						-		-					− −199.3		27.7
	199.3	7	100/0.4 60/0.0								-	- 60/0.0 100/0.4	'			NON-CRYSTALLIN Brown and Gray, Fresh		
195		‡									1 .				薑	Weathered, Very Close to Spacing, Medium Hard to	Close Fracture	
100	-	‡									1:				罿	Siltstone	Tiara, Triadolo	
		‡									:				薹	REC. = 879 RQD = 589		
190		‡							-		<u> </u>					_ GSI = 45-5		
		‡													鼜	- -		
185		‡									:				麗	- -		
100	-	‡									1:				罿	<del>-</del> -		
		‡									:				薹	- -		
180		‡							-		<u> </u>					<u>-</u>		
		‡—	-			┦			-				-			- 178.4 Boring Terminated at Elev	ration 178.4 ft in	48.6
		‡														Non-Crystalline Rock (Tri	assic Siltstone)	
	-	‡														<del>-</del> -		
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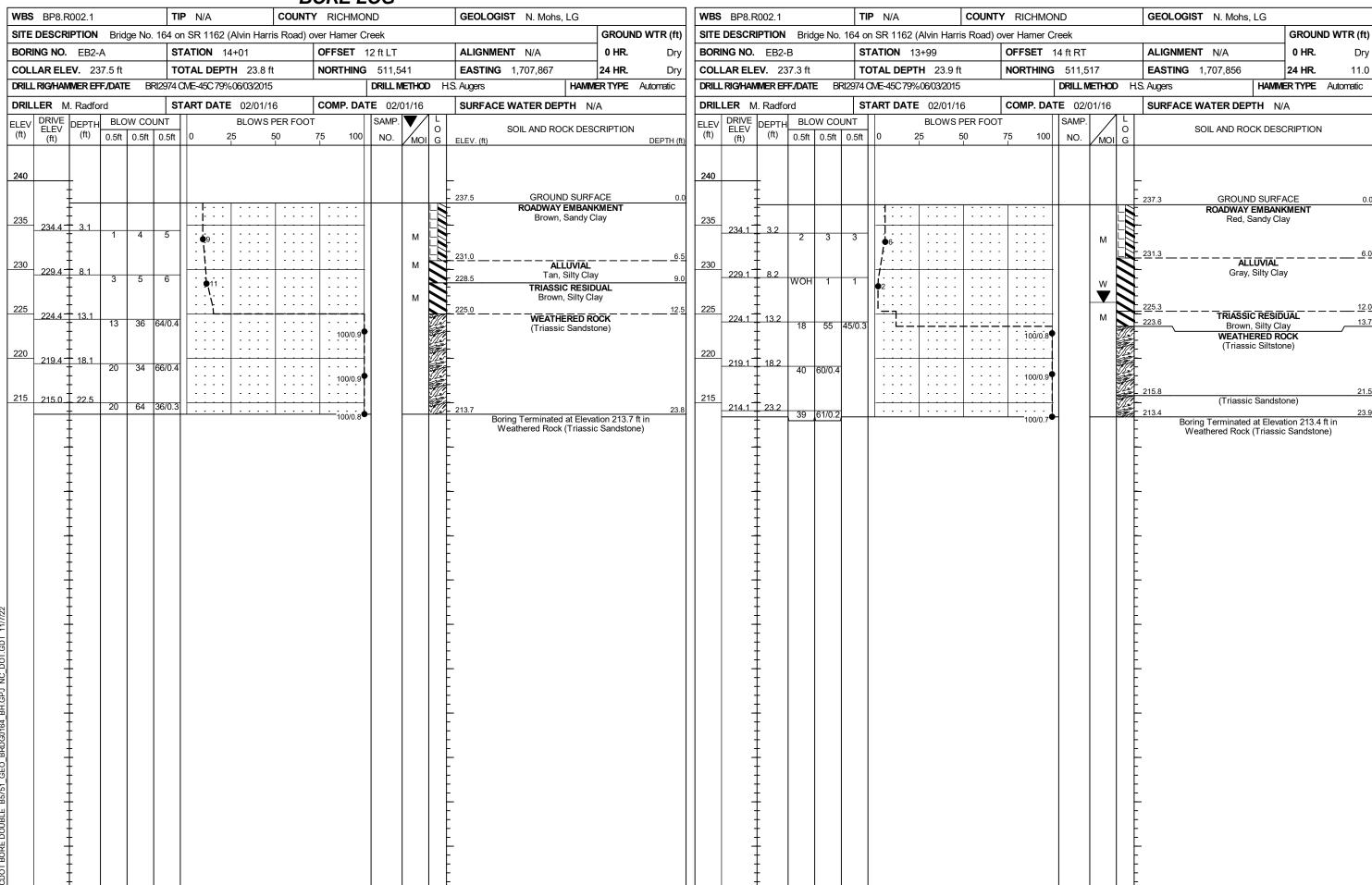
### GEOTECHNICAL BORING REPORT CORE LOG

									<u></u>	<u>UI</u>	RE LOG				
WBS	BP8.F	R002.1			TIP	N/A		C	OUNT	Y F	ICHMOND	GEOLOGIST Kebea,	B.		
SITE	DESCR	IPTION	<b>I</b> Brid	ge No. 1	64 on	SR 11	62 (Alvin	Harris	Roac	) ov	r Hamer Creek			GROUN	ID WTR (ft)
BOR	ING NO	. B1-A	ı		STAT	ΓΙΟΝ	13+65			OF	SET 9 ft LT	ALIGNMENT -L-	0 HR.	N/A	
COL	LAR EL	<b>EV.</b> 22	27.0 ft		TOT	AL DE	<b>PTH</b> 48	.6 ft		NO	<b>RTHING</b> 511,550	<b>EASTING</b> 1,707,832		24 HR.	23.0
DRIL	RIG/HA	MMER E	FF./DA	TE SME	573 CN	ΛE-550)	X 82% 05/	11/2022	2		DRILL METHOD NV	V Casing w/ Advancer	HAMM	ER TYPE	Automatic
DRIL	LER L	ittle, J.			STAF	RT DA	<b>TE</b> 10/1	2/22		CO	<b>MP. DATE</b> 10/13/22	SURFACE WATER DE	PTH N	/A	
COR	E SIZE	NQ					<b>N</b> 20.9 f								
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	REC. (ft) %	JN RQD (ft) %	SAMP. NO.	STR REC. (ft) %	RATA RQD (ft) %	L O G	ELEV. (ft)	DESCRIPTION AND REMARI	KS		DEPTH (ft)
199.3		07.7			(5.5)	(= =)						Begin Coring @ 27.7 ft			
195	199.3 197.4 192.4	† † †	5.0	2:30/0.9 2:30 2:15 3:00 2:45 2:30 3:30 2:15	(0.0) 0% (5.0) 100%	(0.0) 0% (1.8) 36%		(18.1) 87%	(12.1) 58%		Brown and Gray, Fracture Spa	NON-CRYSTALLINE ROC Fresh to Completely Weathere acing, Medium Hard to Hard, GSI = 45-50	d, Very C	lose to Clos iltstone	27.7 se
190	187.4	39.6		2:15 1:30 1:30 2:15	82%	66%				蓋蓋	- - -				
185		† -	5.0	2:15 3:00 4:00 3:30	(5.0) 100%	(3.0) 60%					- - -				
180	182.4	44.6	4.0	3:30 2:00 1:30 3:00 2:15	(4.0) 100%	(4.0) 100%					178.4				48.6
												Siltstone)			

									E	BORE L	0	G						
										Y RICHMOND GEOLOGIS					GEOLOGIST Kebea, B.			
SITE	DESCR	IPTION	<b>I</b> Brid	dge No				-	arris Roa	d) over Hame						-	D WTR (ft)	
BORING NO. B1-B STATION 13+78										OFFSET 2					ALIGNMENT -L- 0 HR. N/A			
								<b>H</b> 56.1		NORTHING					<b>EASTING</b> 1,707,840	24 HR.	19.0	
DRILL RIG/HAMMER EFF./DATE SME6573 CME-55										1				) N\		IER TYPE	<b> </b>	
	LER L DRIVE		T 51.6			ΓAI	RT DATE			COMP. DA	_			L	SURFACE WATER DEPTH N	/A		
ELEV (ft)	ELEV	DEPTH (ft)	0.5ft	0.5ft			) 2	BLOWS 5	PER FOO	75 100		AMP. NO.	'/	0	SOIL AND ROCK DES	CRIPTION		
	(ft)		0.511	0.510	0.511	H		ĭ	<u> </u>	1,0	╁	10.	/MOI	G	ELEV. (ft)		DEPTH (ft)	
005																		
235		‡													<u>-</u>			
		‡													· 231.5 GROUND SURF	ACE	0.0	
230	230.5	1.0	3	4	4	F							М		ROADWAY EMBAN Brown, Silty C			
	228.0	3.5	I WOL	WOH	1	,	, <b>/</b> *								228.5 ALLUVIAL		3.0	
225	225.5	6.0					1						Sat.		226.0 Gray, Highly Plastic, TRIASSIC RESII		5.5	
223	1 -	İ	3	3	9		12						Sat.		Brown and Red, Clay	ey Sand		
	223.0	8.5	8	12	17			0 10					Sat.		Gray and Red, Highly Pla  Brown and Red, Sa		<u>y</u>	
220	220.5	† 11.0 †	5	19	38	╁							Sat.	-	· =			
	:	‡							,						· 217.5		14.0	
215	216.0	15.5	8	9	17			],/					0-4		Red and Brown, Si	ty Clay		
2.0	-	‡						26					Sat.	3477	214.0 WEATHERED R	OCK	17.5	
	211.0	20.5													(Triassic Siltsto			
210	- 211.0	- 20.0	46	54/0.3		┞				100/0.8	•				· <del>-</del>			
		Ŧ													•			
205	206.0	25.5	39	54	46/0.3	$\  \ $									•			
	-	Ŧ								. 100/0.8	'				<del>-</del> ·			
	201.0	30.5													•			
200		<del></del>	64	36/0.2		╟				100/0.7	•				· <del>-</del>			
		Ŧ																
195	196.0 195.6	35.5 35.9	100/0.4	4							,			The state of the s	- 195.6 NON-CRYSTALLIN	F ROCK	35.9	
		1	60/0.0							. 100/0.4					Brown and Gray, Fresh to Weathered, Very Close to	<ul> <li>Completely</li> </ul>	ıre	
400		‡													Spacing, Medium Hard to Siltstone			
190	-	‡				╟									- REC. = 74%			
		‡													RQD = 46% GSI = 45-50			
185	_	‡				┞									. GSI = 45-50 -			
	:	‡																
180		‡															9/23	
=	-	‡							1						<del>-</del>		CD TOORE DOUBLE RICHMOND_164.GPJ NC_DOT.GDT 1/19/23	
5		‡													•		T.GD	
2	_	‡—				Н	• • • •				-				- 175.4 Boring Terminated at Eleva		56.1 O	
2		‡												F	Non-Crystalline Rock (Tria	ssic Siltstone	e) $\vec{z}$	
		‡												F	•		64.GF	
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## GEOTECHNICAL BORING REPORT CORE LOG

											RE LOG		
WBS	BP8.F	R002.1			TIP	N/A		C	OUNT	Υ	RICHMOND	GEOLOGIST Kebea, B.	
SITE DESCRIPTION Bridge No. 164 on SR 1162 (Alvin Harris Ro												,	GROUND WTR (ft)
BOR	ING NO	. B1-B	<b>,</b>		+		13+78			OF	FSET 2 ft RT	ALIGNMENT -L-	<b>0 HR.</b> N/A
	LAR EL						<b>PTH</b> 56			NC	<b>RTHING</b> 511,535	<b>EASTING</b> 1,707,840	<b>24 HR.</b> 19.0
			FF./DA	TE SME			X 82% 05/		2	_	DRILL METHOD NV		AMMER TYPE Automatic
	LER L				<u> </u>		<b>TE</b> 10/1			CC	<b>MP. DATE</b> 10/12/22	SURFACE WATER DEPTH	N/A
	E SIZE RUN	1		DRILL		AL RU Un	<b>N</b> 20.2 f		ATA	-			
ELEV (ft)	ELEV (ft)	DEPTH (ft)	RUN (ft)	RATE (Min/ft)	REC. (ft) %	RQD (ft) %	SAMP. NO.	REC. (ft) %	RQD (ft) %	O G	ELEV. (ft)	DESCRIPTION AND REMARKS	DEPTH (ft)
195.6 195	195.6 -	35.9	3.2	0:15/0.2	(1.6)	(0.0)		(14.9)	(9.2)		<del></del>	Begin Coring @ 35.9 ft NON-CRYSTALLINE ROCK	35.9
	192.4	39.1		0:15/0.2 1:15 1:30 1:30	50%	0%		74%	46%		<ul> <li>Brown and Gray, F</li> </ul>	resh to Completely Weathered, Ve cing, Medium Hard to Hard, Triass	ery Close to Close
190		+	5.0	2:00 4:30	(2.2) 44%	(0.0)					<u>.</u> -	GSI = 45-50	
130	-	‡		4:15 3:45							_ -		
	187.4	44.1	5.0	4:15 4:30	(4.1)	(3.3)					- -		
185	_	‡		3:30 4:30	82%	66%					<u>-</u>		
	182.4	49.1		3:00 3:00	(= -)						<u>-</u>		
180		Ī	5.0	2:45 4:00	(5.0) 100%	(4.1) 82%					_ _ _		
	177.4	T 54.1		4:15 4:00									
	175.4	56.1	2.0	3:30 2:30 2:30	(2.0) 100%	(1.8) 90%					_ - 175.4		56.1
	- 175.4	50.1		2.30	100%	90%						at Elevation 175.4 ft in Non-Crystal Siltstone)	
		‡									<u>-</u> -	Silisione)	
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### **CORE PHOTOGRAPHS**

**B1-A**BOXES 1 - 2: 29.6 – 48.6 FEET

NR-FIRST RUN 39.6 39.6° FEET

**B1-B**BOXES 1 - 2: 35.9 – 56.1 FEET





## **SITE PHOTOGRAPH**

Bridge No. 164 on -L- (SR 1162) over Hamer Creek



Looking Northwest towards End Bent 1